



# Hydraulic structures. Dams and reservoirs

## Concrete dam engineering -1

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**Strengthening of master curricula in water resources  
management for the Western Balkans HEIs and stakeholders**

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# DAM BODY GEOMETRY

## *Elevations:*

- Dam Crest El.
- Top of a Basic Triangle El. = NWL / MWL
- Foundation El. =  $f$  (River Bed El, geology)

*Upstream Slope 1:m*

*Downstream Slope 1:n*

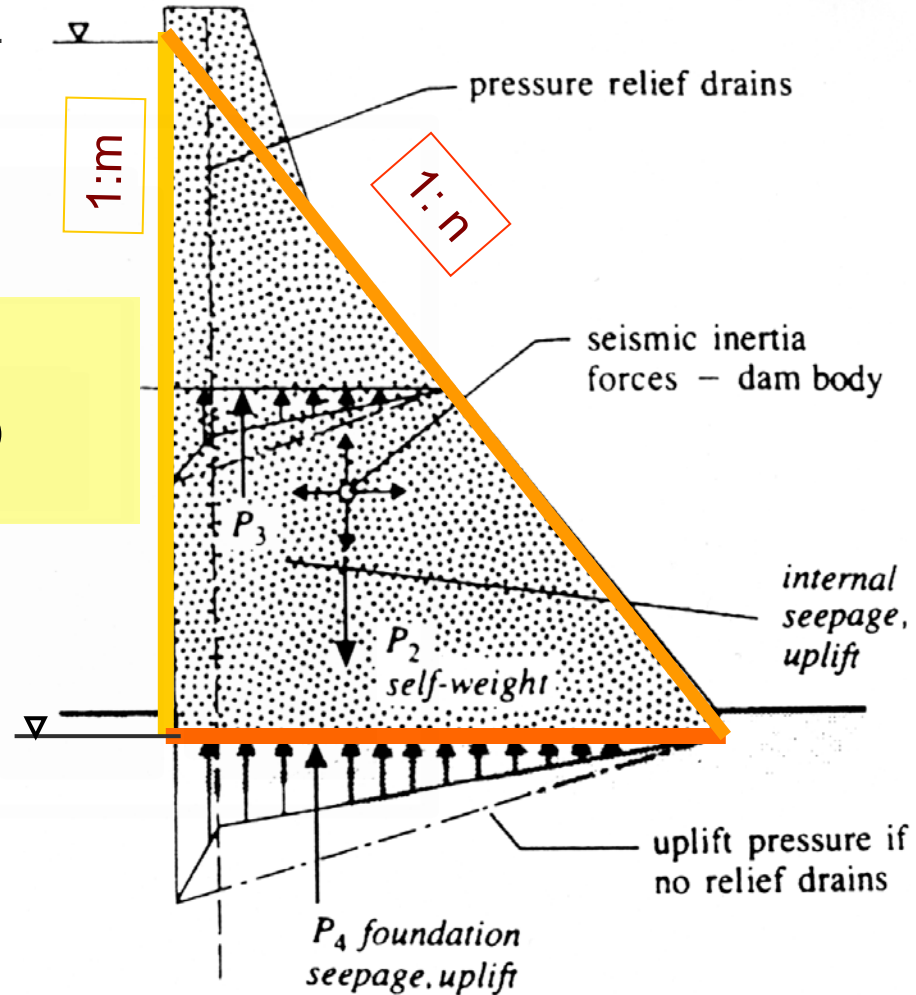
# DAM BODY GEOMETRY

Top of a Basic Triangle El.= NWL / MWL

Dam body geometry

- 1:m,  $m=0.05\dots0.10$   
1:n,  $n=1.68\dots1.75$

Foundation El.= River Bed El.-(A1+WRock)



# LOADS ON DAM

- ***primary loads:*** water and related seepage loads, self-weight loads;
- ***secondary loads:*** sediment load, hydrodynamic wave load, ice load
- ***exceptional loads:*** seismic load

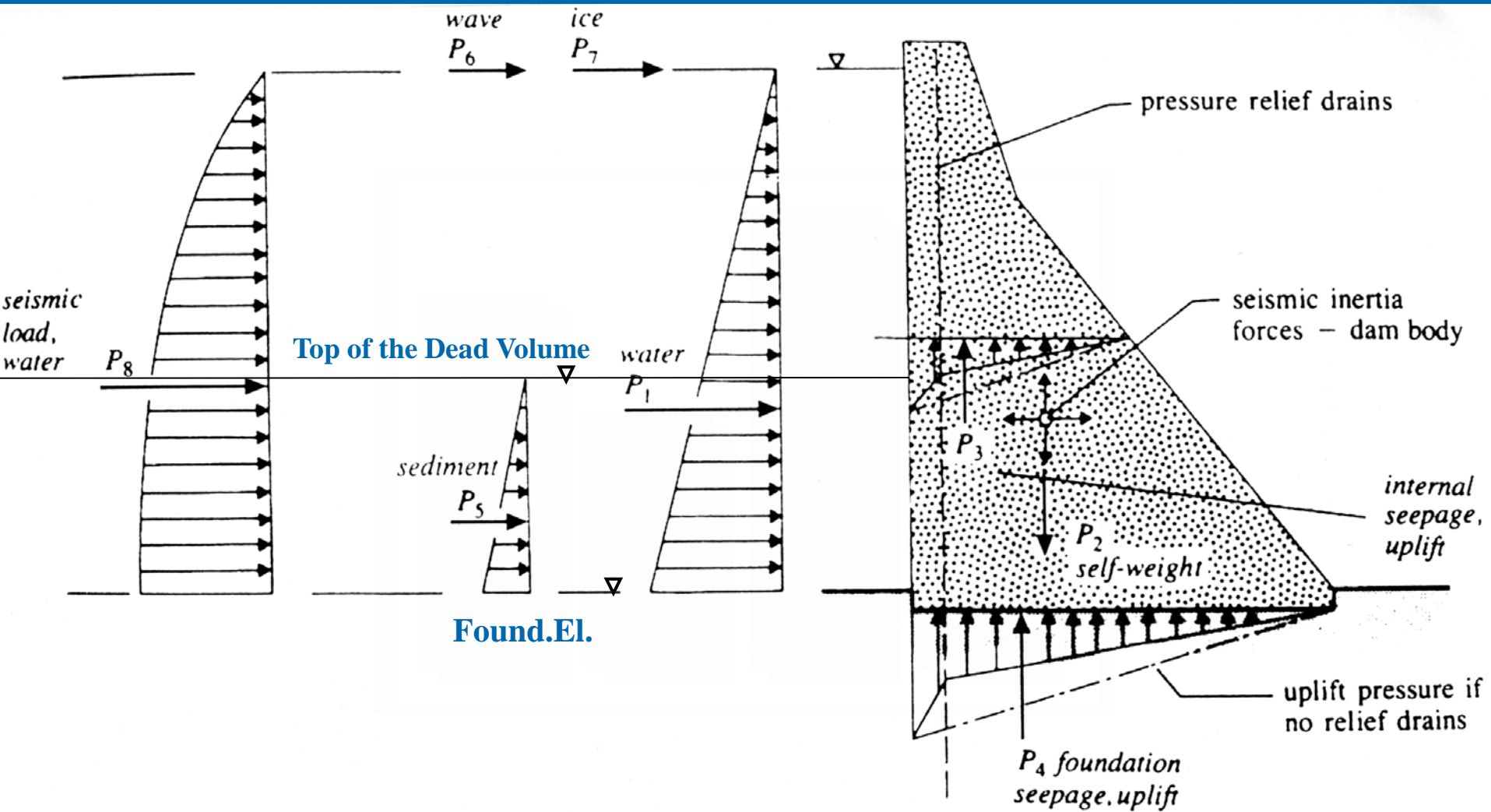
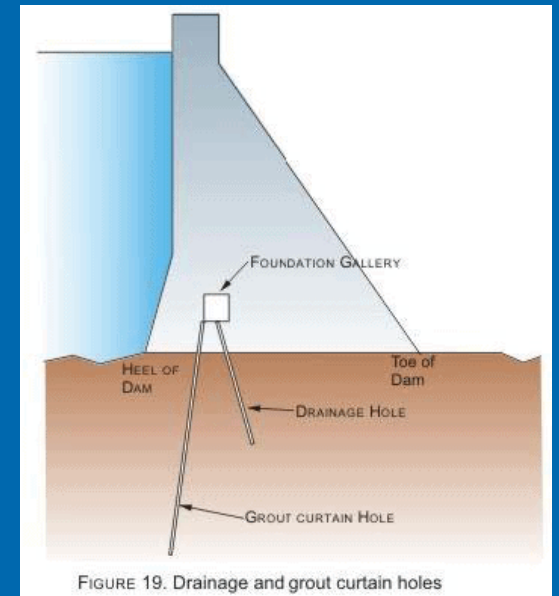


Fig. 1.8 Schematic of principal loads: gravity dam profile

# Seepage control

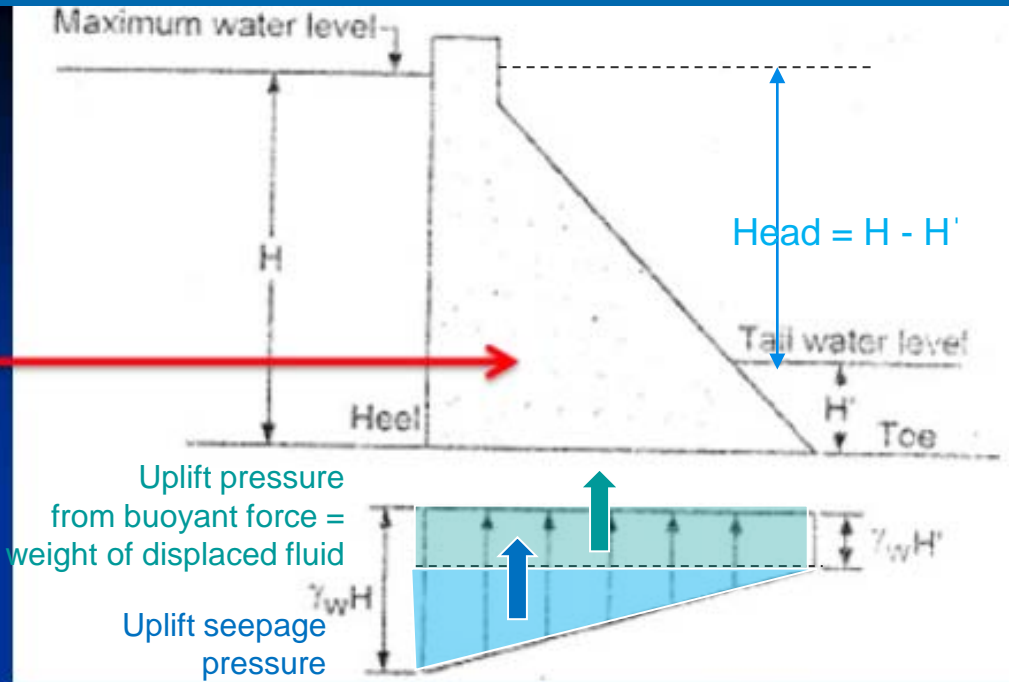
## Galery

- ease of construction and rehabilitation of the grout curtain and drains in foundation
- seepage water collection



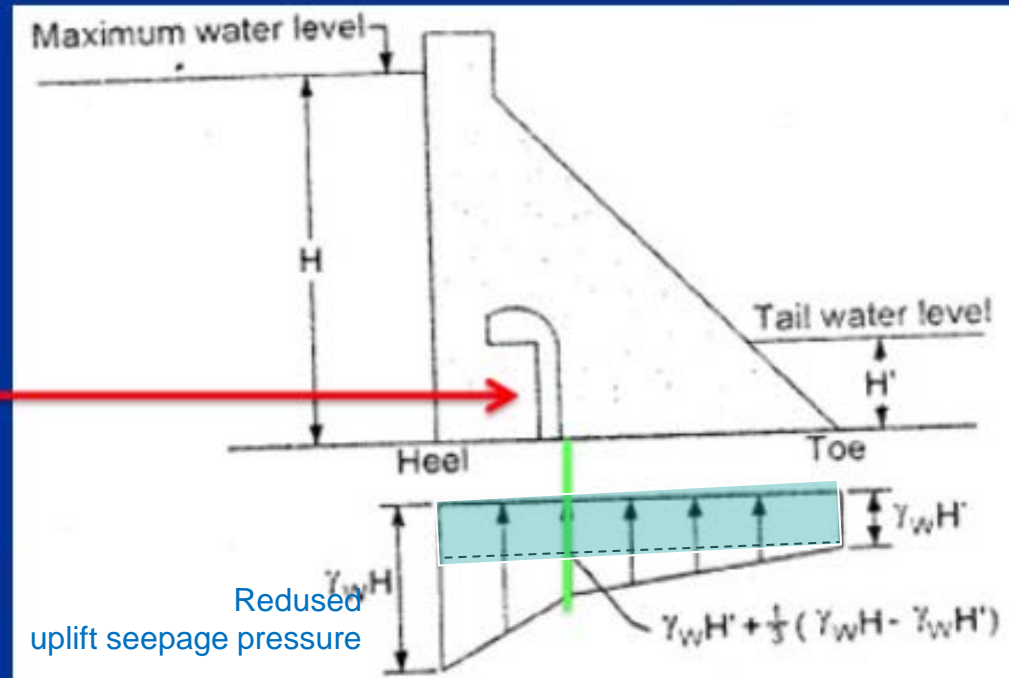
# Uplift Pressure

## Without Gallery



Uplift pressure from Archimedes' principle: buoyant force = weight of displaced fluid

## With Gallery



Uplift seepage pressure

$$\gamma_w H' + \frac{1}{3} (\gamma_w H - \gamma_w H')$$

Reduced uplift seepage pressure



# LOAD COMBINATIONS

## *I. End of Construction :*

Normal = G

**Extreme** = G + **E**

## *II. Operation :*

Normal :  $\nabla WL = \nabla NWL, G \quad W_H \quad W_V \quad P_H \quad W_\phi$

**Unusual**:  $\nabla WL = \nabla MWL, G \quad W_H \quad W_V \quad P_H \quad W_\phi$

**Extreme**:  $\nabla WL = \nabla NWL, G \quad W_H \quad W_V \quad P_H \quad W_\phi + \mathbf{E}$



# LOADS ON DAM

- *primary loads*: water and related seepage loads, self-weight loads;
- ***secondary loads***: sediment load,
- ***exceptional loads***: seismic load

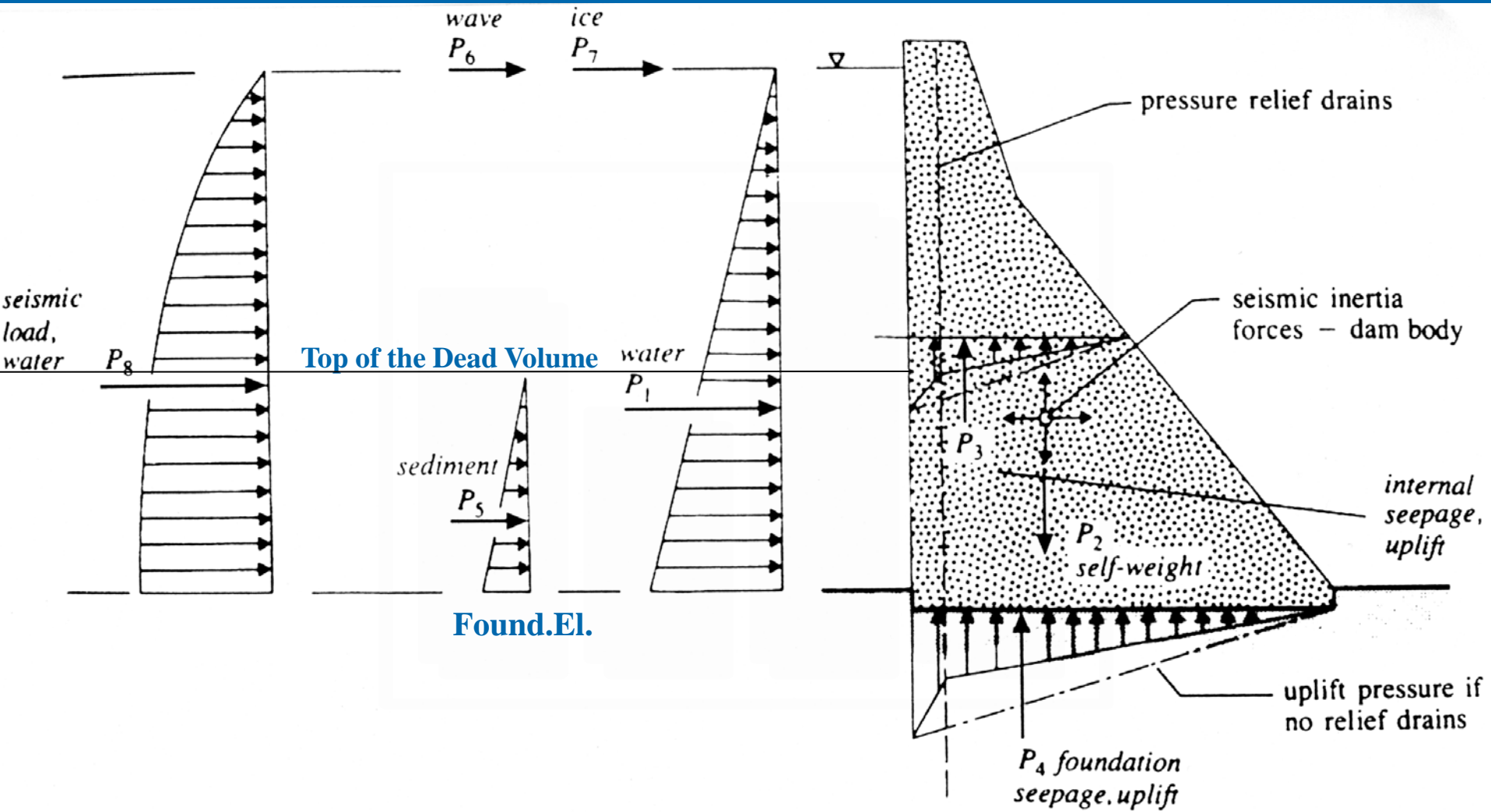


Fig. 1.8 Schematic of principal loads: gravity dam profile

# THEORETICAL VALUES OF LOADS

## Sediment Load

$$P_H = \frac{1}{2} \cdot \gamma_H \cdot H_H^2 \cdot \xi_a \quad (1) \quad H - \text{from Top of the Dead Volume to Found.El.}$$

Dry Unit Weight:

$$\gamma_1 = 12 \div 14 \text{ kN/m}^3$$

$$\gamma_H = \gamma_1 - (1 - n) \cdot \gamma_B$$

$$\gamma_H = 13 - (1 - 0,34) \cdot 10 = 6,40 \text{ kN/m}^3$$

For Submerged Unit Weight :

$\gamma_1 + n \cdot \gamma_w$  -> the sediment saturated unit weight

$\gamma_H$  -> weighed sediment unit weight

Example

Where: n - pore space

$$\xi_a = \text{tg}^2 \left( 45^\circ - \frac{\varphi}{2} \right) - \text{Active Lateral Pressure Coefficient}$$

$\varphi = 16^\circ \div 18^\circ$  - Angle of Shearing Resistance of the Sediment

Example

$$\xi_a = \text{tg}^2 \left( 45^\circ - \frac{17^\circ}{2} \right) = 0,55$$

$$H_H = \nabla MO - \nabla \Phi = 1052,60 - 1021,20 = 31,40 \text{ m}$$

$$P_H = \frac{1}{2} \cdot 6,40 \cdot 31,40^2 \cdot 0,55 = 1735,29 \text{ kN/m}^1$$

# LOADS ON DAM

- *primary loads*: water and related seepage loads, self-weight loads;
- *secondary loads*: sediment load,
- *exceptional loads*: seismic load

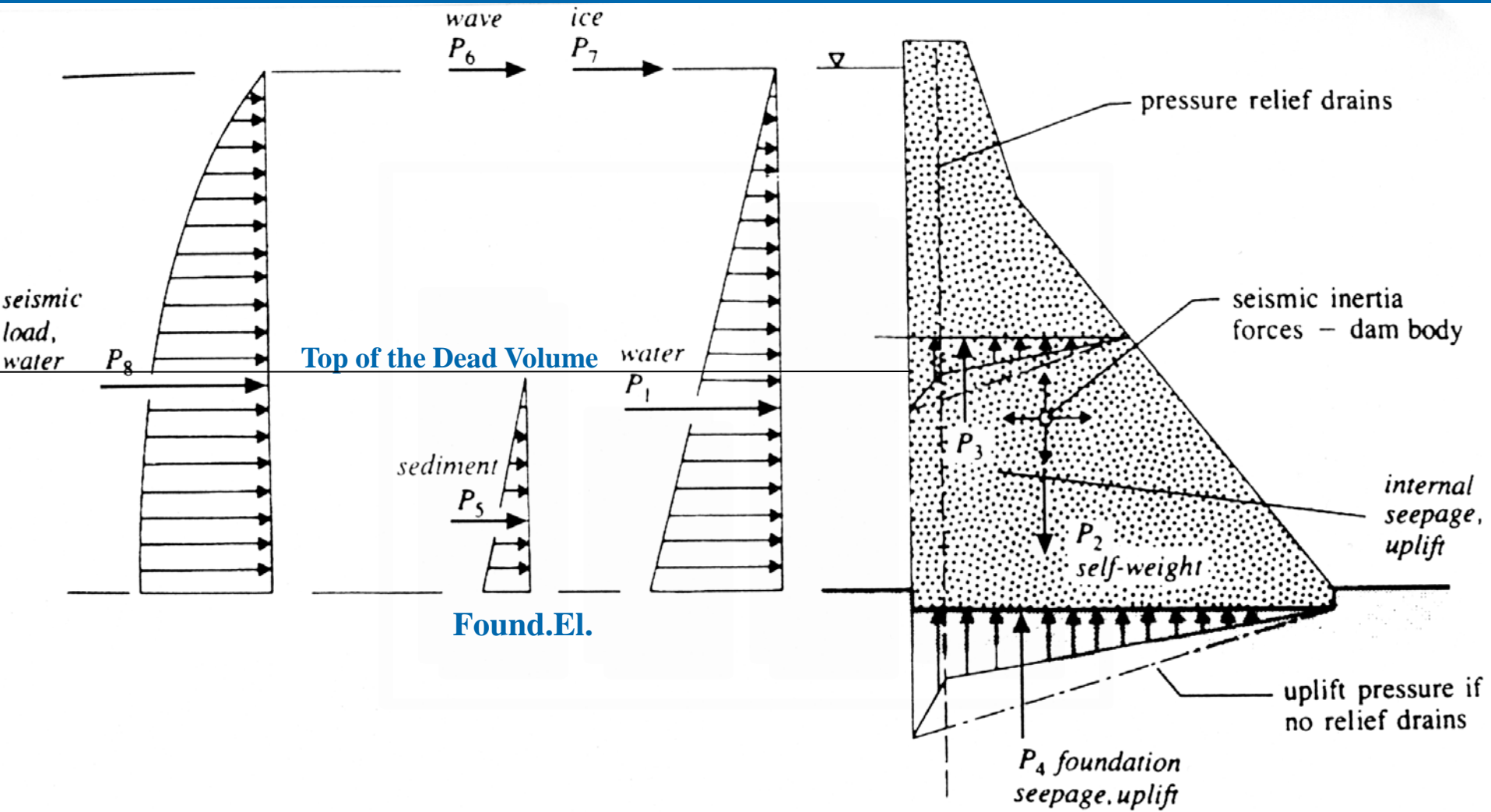
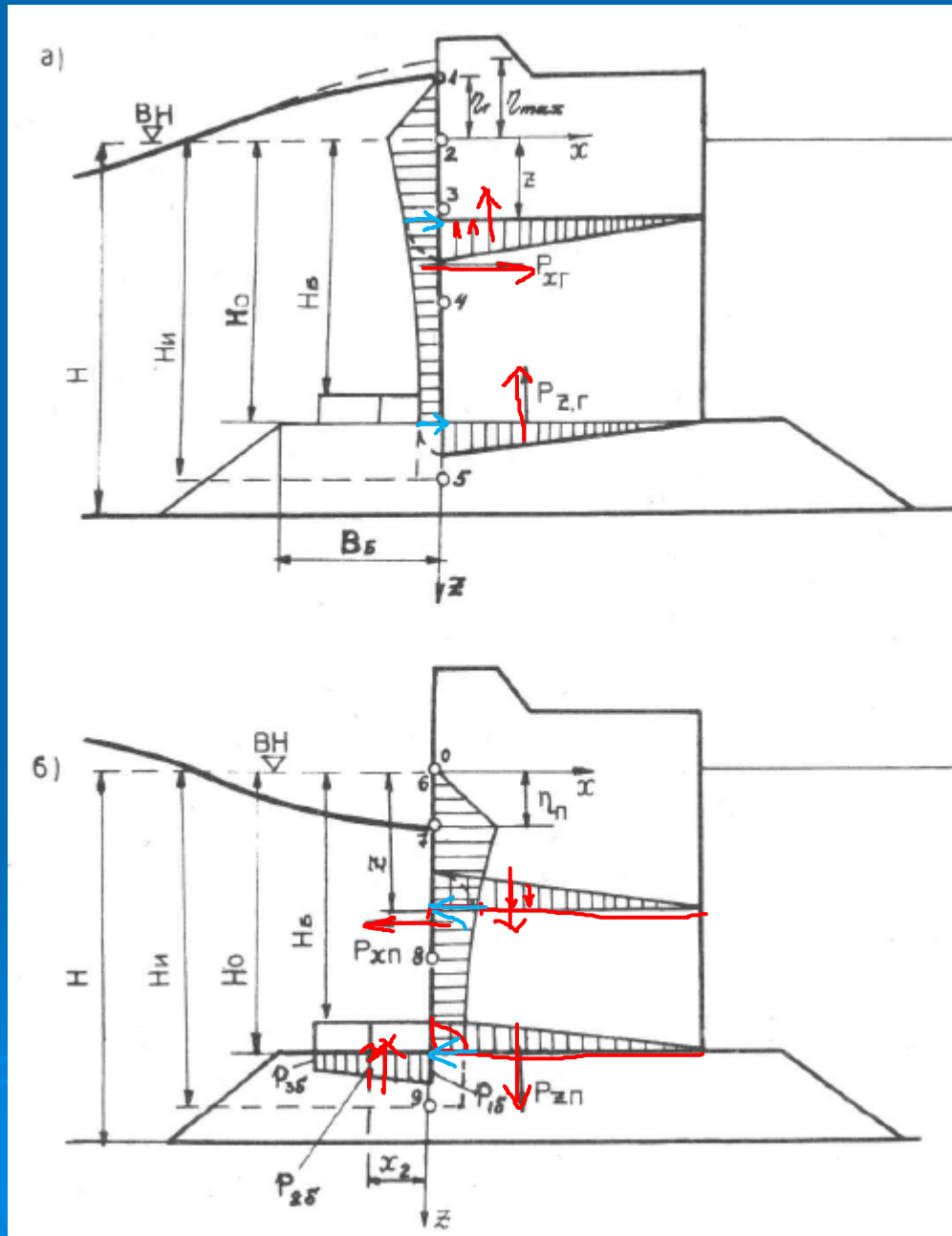


Fig. 1.8 Schematic of principal loads: gravity dam profile

# Loading by wind wave



***exceptional loads:*** seismic

# Response-Spectrum Analysis

- We seek **maximum response** of the structure to the ground motion  
/response quantities: displacements, forces and stresses/

***MAX EARTHQUAKE***

***CME /controlling max earthquake/***



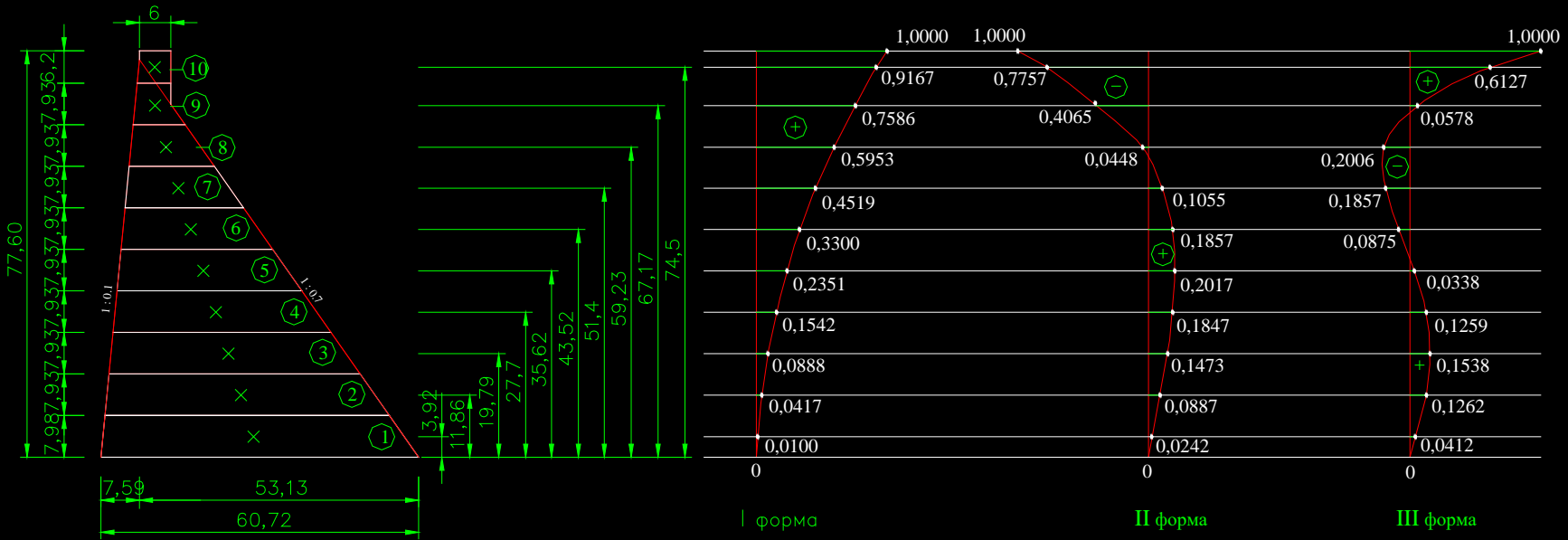
# Response-Spectrum Method

In Spectral method seismic inertia loads are determined by modes of free oscillations  $i=1,2,3,4,\dots$

Design Earthquake Force  
/Bulgarian code/

$$E_{ik} = c R K_c \beta_i \eta_{ik} Q_k$$

# Modes of free oscillations depend on B/H dam value



$$E_k = \sqrt{\sum_i E_{ik}^2}$$

$$E_{ik} = cRK_c \beta_i \eta_{ik} Q_k$$

Where:

$Q_k$  – weight of “κ” lamella

$$Q_k = m \cdot g$$

$c$  - coefficient of responsibility  
 $c=1.5$  for I class dam

$R$  - coefficient of material and type of structure  
 $R=0.3$  for concrete gravity dam

$K_c$  - earthquake coefficient / from the Earthquake map of the country  
 $K_c = a/g = 0.1; 0.15; 0.27$  / for VII; VIII; IX degree /

$\beta_i$  - acceleration response  
 versus the period  $T_i$

$$\eta_{i,k} = X_{i,k} \cdot \frac{\sum_1^n Q_j \cdot X_{i,j}}{\sum_1^n Q_j \cdot X_{i,j}^2}$$

-coefficient of the shape for “i”-mode on level “k”

## Algorithm:

1. Calculate the period of structure's free vibrations in modes 1,2,3 and the corresponding forms of free oscillations for  $B/H=???$
2. Calculate coefficients  $\beta, \eta$
3. Calculate seismic loads in modes 1,2,3
4. Calculate combination = total seismic load

# SEISMIC LOAD

## *I. End of Construction :*

**Extreme = G + E**

$$E_{ik} = cRK_c \beta_i \eta_{ik} Q_k$$

## *II. Operation :*

**Extreme:  $\nabla WL = \nabla NWL, G W_H W_V W_\phi + E$**

$$E_{ik} = cRK_c \beta_i \eta_{ik} (Q_k + Q_{wk})$$

# Operation

$$E_{ik} = cRK_c \beta_i \eta_{ik} (Q_k + Q_{wk})$$

## ATTACHED MASS OF WATER

We replace the hydro-seismic pressure by adequate ATTACHED to the dam  
MASS OF WATER.

Then we have to calculate NEW free oscillations of the dam-water system.

